A34 A35 — Reinforced Angle Bracket

Material: Carbon Steel 1.5mm thick

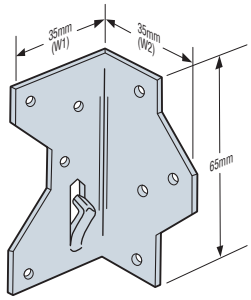
Finish:
Z275 Galvanised: A34; A35

Corrosion Resistance Level
LOW

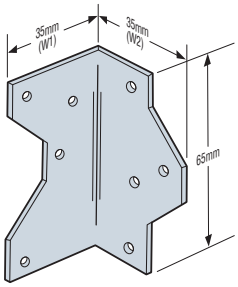
316 Stainless Steel: A34SS; A35SS

Corrosion Resistance Level
SEVERE

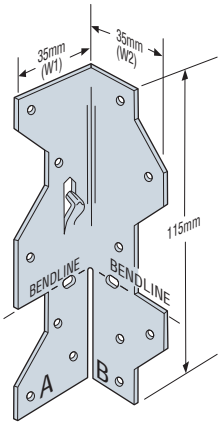
Size: See illustration below



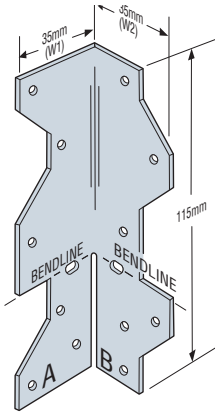
A34



A34SS



A35



A35SS

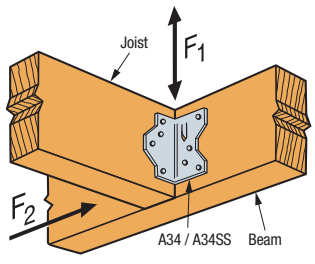
Features & Benefits

- Removes the guesswork for making perfectly square connections
- Reinforces 90-degree connections
- Stronger than angled nailing or screw fastening
- Staggered fastener pattern reduces the chances of timber splitting
- Speed Prong feature helps to temporarily position and secure the connector for easier and faster installation
- Bending slot allows field bends for two and three-way tie connections
- Manufactured in heavier gauge steel for a stronger connection

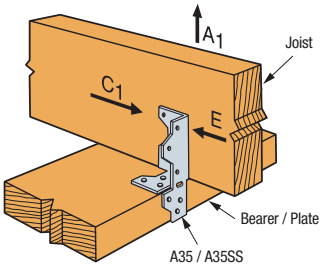
Installation

- Use all specified fasteners
- A35 — bend one time only

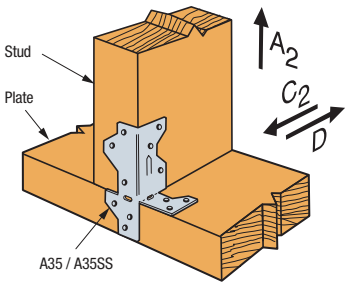
Construction Details



A34/A34SS Joist-to-Beam



A35/A35SS Joist-to-Bearer/Plate



A35/A35SS Stud-to-Plate

Simpson Strong-Tie® (New Zealand) Ltd
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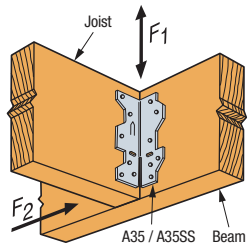
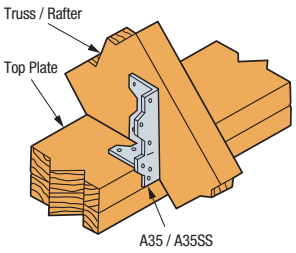
www.strongtie.co.nz

A34 A35 — Reinforced Angle Bracket



A35/A35SS Joist-to-Beam



A35/A35SS Truss/Rafter-to-Top Plate

A34 and A35 Technical Data

Model No.	Dimension (mm)		Type of Connection	Fasteners (No. – Length x Dia., mm)	Direction of Load	Design Capacity (kN) Australia			Design Capacity (kN) New Zealand		
	W1	W2				Floor k_1 = 0.69	Roof k_1 = 0.77	Wind/EQ k_1 = 1.14	Floor k_1 = 0.80	Roof k_1 = 0.80	Wind/EQ k_1 = 1.0
A34	35	35	1	8 – 38 x 3.32	F1	0.99	0.99	0.99	0.93	0.93	0.93
					F2[7]	0.95	0.95	0.95	0.90	0.90	0.90
A34SS	35	35	1	8 – 38 x 3.32	F1	0.67	0.67	0.67	0.63	0.63	0.63
					F2[7]	0.65	0.65	0.65	0.61	0.61	0.61
A35	35	35	2	9 – 38 x 3.32	A1, E	1.40	1.47	1.47	1.21	1.21	1.38
					C1	0.44	0.44	0.44	0.42	0.42	0.42
			3	12 – 38 x 3.32	A2	1.40	1.57	1.68	1.21	1.21	1.51
					C2	0.86	0.86	0.86	0.81	0.81	0.81
					D	1.05	1.05	1.05	0.99	0.99	0.99
			4	12 – 38 x 3.32	F1	2.21	2.21	2.21	2.08	2.08	2.08
					F2[7]	1.43	1.43	1.43	1.34	1.34	1.34
A35SS	35	35	2	9 – 38 x 3.32	A1, E	0.88	0.88	0.88	0.83	0.83	0.83
					C1	0.31	0.31	0.31	0.29	0.29	0.29
			3	12 – 38 x 3.32	A2	1.26	1.26	1.26	1.18	1.18	1.18
					C2	0.65	0.65	0.65	0.61	0.61	0.61
					D	0.84	0.84	0.84	0.79	0.79	0.79
			4	12 – 38 x 3.32	F1	1.76	1.76	1.76	1.66	1.66	1.66
					F2[7]	1.14	1.14	1.14	1.08	1.08	1.08

1. Design Capacity is the lesser of (1) the Characteristic Capacity multiplied by the Australian Capacity Factor, or the NZ Strength Reduction Factor (ϕ), and applicable the k modification factors following AS 1720.1 and NZS 3603 and (2) the Serviceability Capacity which is the load at 3.2mm joint slip. Design Capacity is the minimum of test data and structural joint calculation.
2. For Australia, the Capacity Factor (ϕ) is 0.85 for nails and screws for structural joints in a Category 1 application. Reduce tabulated values where other Category applications govern. For NZ, the Strength Reduction Factor (ϕ) is 0.80 for nails in lateral loading.
3. Duration of Load Factor (k_1) is as shown. Reduce Duration of Load Factor where applicable. Capacities may not be increased.
4. Timber species for joint design is seasoned Radiata Pine, which is Australia Joint Group JD4 per AS 1720.1 Table H2.4 and New Zealand Joint Group J5 per NZS 3603 Table 4.1.
5. Design capacities are for one part. When parts are installed on each side of the joist, the minimum joist beadth is 75mm.
6. Some illustrations show connections that could cause cross–grain tension or bending of the timber during loading if not reinforced sufficiently. In this case, mechanical reinforcement should be considered.
7. Connectors are required on both sides to achieve F2 load in both directions.
8. For simultaneous loads in more than one direction, the connector must be evaluated using the Unity Equation. See General Note 'e' on page 15.

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